

Development of a Coupled Approach for Structural Damage Detection with Incomplete Measurements

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A procedure has been developed that couples model order reduction, damage location, dynamic residual and mode shape expansion, and damage extent estimation to overcome the incomplete measurements problem by using an appropriate undamaged structural model. The first significant contribution of this work is in the development of a process to estimate the full dynamic residuals using the columns of a spring connectivity matrix obtained by disassembling the structural stiffness matrix. The second significant contribution is the extension of an eigenvector filtering procedure to produce full-order mode shapes that more closely match the measured active partition of the mode shapes using a series of modified Ritz vectors. The full dynamic residuals and full mode shapes are used as inputs to the minimum rank perturbation theory to provide an estimate of damage location and extent. An analytical model of the NASA eight-bay truss and an experimental data set are used to exercise this process and to study the sensitivity to various parameters.

Nomenclature

$\{b\}, [B]$	= dynamic residual vector and dynamic residual matrix, respectively
$[C], [P]$	= disassembly connectivity matrix and disassembly magnitude matrix, respectively
$[D]$	= dynamic stiffness matrix
$\{F\}$	= force vector
$[K], [\Delta K]$	= stiffness matrix and stiffness matrix change, respectively
$[M], [\Delta M]$	= mass matrix and mass matrix change, respectively
$[T]$	= reduction/expansion transformation matrix
$\{U\}$	= displacement vector
α	= expanded mode shape vector coefficient
$\{\beta\}$	= reduced dynamic residual vector
$\gamma, [V]$	= basis vector coefficient and basis vector matrix, respectively
$\{\nu\}, \{\bar{\nu}\}$	= modified Ritz vector and nonorthogonal/nonnormal Ritz vector, respectively
$\{\phi\}, [\Phi]$	= mode shape vector and mode shape matrix, respectively
ω	= modal frequency

I. Introduction

THE use of changing structural dynamics parameters as indicators of damage state and structural health has received increasing attention in recent years. The global nature of these parameters provides a unique capability to monitor internal components of aerospace, civil, mechanical, naval, and nuclear structures. The ability to model the structural response analytically enhances this unique capability to locate damage in unmeasured locations as well as to provide tools to determine the extent of the damage. However, much work needs to be done to use experimental and analytical models properly in a coordinated fashion to perform damage identification. One of the most difficult issues to overcome is the incomplete measurement problem, which means that 1) there is an

inherent mismatch between the experimental and the analytical degrees of freedom (DOFs) and 2) there is a mismatch between the numbers of measured and analytical modes. Another difficult issue is that neither the experimental measurements (noise) nor the analytical model (modeling errors) is correct. The last statement is especially true in damage identification as the analytical model is typically produced for the undamaged structure.

The incomplete measurement problem means that model order reduction¹⁻⁶ and/or test data expansion⁴⁻¹¹ must be performed to utilize both data sets fully. However, it has been found that most reduction/expansion techniques are not extremely useful for damage identification.^{6,10,11} It has been suggested that reduction/damage identification/expansion should be handled as coupled problems.⁶ This work describes the development of such an integrated procedure that is initiated by first disassembling the structural stiffness matrix¹²⁻¹⁵ into a connected set of springs. The columns of the spring connectivity matrix are then used as a set of candidate basis vectors for estimating the full dynamic residual. This candidate set is down selected using the modal assurance criteria (MAC) between the reduced dynamic residual and a reduction of each candidate vector averaged over the modes of interest. A least-squares fit is then used to scale the remaining basis vectors to match the dynamically reduced residuals for each mode. The estimated full residuals provide damage location information and are used to initiate a modified Ritz vector¹⁶ calculation process. A set of Ritz-like vectors are calculated and used as a set of basis vectors to expand the mode shapes and to guarantee mass orthogonality. The full dynamic residuals and full mode shapes are then used as inputs to the minimum rank perturbation theory (MRPT)¹⁷⁻¹⁹ to provide an estimate of damage.

II. Technical Background

This section briefly describes the basic theoretical components used in the coupled damage detection approach.

Dynamic Residual Formulation

This study will proceed with a standard math model for a dynamical system (ignoring damping):

$$[M]\{\ddot{U}\} + [K]\{U\} = \{F\} \quad (1)$$

The corresponding eigenvalue problem for the undamaged structure is given as

$$[M\varpi_u^2 + K]\{\phi_u\} = 0 \quad (2)$$

where the subscript u denotes undamaged and i denotes the i th mode of vibration. The eigenvalue problem for the damaged structure is

$$[(M - \Delta M)\varpi_i^2 + (K - \Delta K)]\{\phi_i\} = 0 \quad (3)$$

Presented as Paper 97-0362 at the AIAA 35th Aerospace Sciences Meeting, Reno, NV, Jan. 6-9, 1997; received March 18, 1998; revision received Aug. 7, 1998; accepted for publication Aug. 13, 1998. Copyright © 1998 by the authors. Published by the American Institute of Aeronautics and Astronautics, Inc., with permission.

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where the Δ matrices represent the effect of damage on the structural property matrices. Now Eq. (3) can be rewritten in dynamic residual form:

$$[M\omega_i^2 + K]\{\phi_i\} = \{b_i\} = [\Delta M\omega_i^2 + \Delta K]\{\phi_i\} \quad (4)$$

Model Reduction

The dynamic residual form can be rearranged into an active or measured set of DOFs and an omitted or unmeasured set of DOFs. It is commonly assumed that the model of the undamaged structure can be used to create a transformation relating the unmeasured and measured DOFs. One approach is to produce a physical-coordinates-based transformation⁷ that for a single mode shape has the following form:

$$\{\phi_i\} = \begin{Bmatrix} \phi_{a_i} \\ \phi_{o_i} \end{Bmatrix} = [T]\{\phi_{a_i}\} = \begin{bmatrix} T_{aa} \\ T_{oa} \end{bmatrix} \{\phi_{a_i}\} \quad (5)$$

The transformation matrix $[T]$ in Eq. (5) can be calculated from the stiffness matrix $[K]$ using the static or Guyan-Irons^{1,2} reduction/expansion. A scaled sum of the mass $[M]$ and stiffness matrices can also be used to estimate $[T]$ as suggested by dynamic reduction/expansion.³

The transformation matrix $[T]$ denoted in Eq. (5) can be used to reduce the order of the analytical model. Substituting Eq. (5) into Eq. (4) and premultiplying by $[T]^T$, one finds that the reduced equations of motion are

$$[T]^T[M\omega_i^2 + K][T]\{\phi_{a_i}\} = \{b_{r_i}\} = [T]^T\{b_i\} \quad (6)$$

The reduced dynamic residual $\{b_{r_i}\}$ given in Eq. (6) has been shown to be of importance in damage identification^{17–20} to localize and to calculate the extent of damage. However, these capabilities are limited by the application of the transformation $[T]$, which tends to redistribute residual forces to undamaged DOFs. A primary contribution of this work is to provide an approach to retain these capabilities by first estimating the full dynamic residual $\{b_i\}$. The procedure suggested to perform this estimation begins by performing a matrix disassembly on the stiffness matrix.^{12–15} This will produce a set of basis vectors used to estimate the full dynamic residual.

Matrix Disassembly

Matrix disassembly is a process that decomposes a structural matrix into a matrix representation of the connectivity between DOFs and a matrix containing the magnitude information.^{12–15} This formulation has the following form for the stiffness matrix:

$$[K] = [CPC^T] \quad (7)$$

Advanced applications utilize a disassembly into the same finite elements that were used to create the model.¹⁴ However, this work utilizes a disassembly into a set of equivalent springs. This produces a general technique that can be applied to any model without detailed knowledge about the actual elements used in the assembly. The matrix $[C]$ is an $n \times m$ matrix, where n is the matrix dimension for $[K]$ and m is equal to the total number of unique entries in $[K]$ (for symmetric $[K]$ this amounts to the nonzero entries in the upper triangular portion of the stiffness matrix). The diagonal matrix $[P]$ is calculated as

$$P(i, i) = \sum_{j=1}^n K(i, j), \quad \text{for } i = 1 : n$$

$$P(i, i) = -K(j, k), \quad \text{for } i = n + 1 : m$$

The first n columns/rows of $[C]$ form an $n \times n$ identity matrix. The remaining $(m - n)$ columns are defined according to the element locations of the unique entries in the matrix $[K]$. For the element $K(j, k)$, which is used to define the i th row/column of $[P]$ for the index i running from $n + 1$ to m , the i th column of $[C]$ is given as

$$C(j, i) = 1.0$$

$$C(k, i) = -1.0$$

Estimating the Expanded Dynamic Residual

For each mode of interest, Eq. (6) relates a known quantity ($\{b_{r_i}\}$, the reduced dynamic residual) to an unknown quantity ($\{b_i\}$, the full dynamic residual). This work assumes that the full dynamic residual can be described as a linear combination of the columns of the connectivity matrix. Therefore, each column will initially be considered a candidate for inclusion in the calculation of the full dynamic residual vector for that mode. The first issue is to down select the set of columns of $[C]$ to the most promising subset for further analysis. Although it was not rigorously necessary, this work constrained the selected subset to have linearly independent columns. (The full $[C]$ matrix contains a high degree of column linear dependence.) Each candidate vector from $[C]$, $\{c_j\}$, will serve as an approximate full dynamic residual vector. The resulting reduced dynamic residual basis vector $\{\beta_{r_{ij}}\}$ is calculated from Eq. (6) by replacing $\{b_i\}$ with $\{c_j\}$. In this work, a comparison with the actual reduced dynamic residual for that mode is made using the MAC, which relates the global similarity between two vectors:

$$\text{MAC}_{ij} = \frac{\|b_{r_i}^T \beta_{r_{ij}}\|^2}{\|b_{r_i}^T b_{r_i}\| \|\beta_{r_{ij}}^T \beta_{r_{ij}}\|} \quad (10)$$

The MAC values are averaged over the modes of interest. These averaged values then provide a discriminator between the candidate columns of the connectivity matrix. Columns $\{c_j\}$, which produce large MAC values over a wide range of modes, are candidates for down selection. The user must select the number of the largest average MAC candidate basis vectors to include in the next analysis. This selection is made based on the relative sizes of the average MAC values, the spatial relationship between the candidate vectors, and the linear dependence of the resulting set of vectors.

The next step is to use the selected vectors as basis vectors for approximating the full dynamic residual. Substituting a linear combination of the basis vectors into Eq. (6) yields

$$\{b_{r_i}\} = [T]^T\{b_i\} = [T]^T[C_{\text{selected columns}}]\{\alpha_i\} \quad (11)$$

where the unknown coefficient α_i is determined by a least-squares approach. (Note that the reduction transformation will change with each mode if dynamic reduction is used.) The critical output of this step is a set of full dynamic residual vectors that match the reduced equations. The next step will be to use these expanded residuals to expand the mode shapes.

Mode Shape Expansion

Mode shape expansion estimates the unmeasured DOFs given the undamaged model, the experimental modal frequency, the measured mode shape DOFs, and the full dynamic residual. However, the physical coordinate transformation approach listed earlier typically assumes that the omitted partition of the dynamic residual is zero.^{1–3} Because this may not be the case for the damage identification problem, this approach must be modified assuming a nonzero residual vector.

An approach called eigenvalue filtering (EV) utilizes the full residual to expand the mode shape. This approach uses Eq. (4) to generate the expansion by inverting the dynamic stiffness matrix $[D]$ using the associated experimental modal frequency ω_i (Ref. 21):

$$\{\phi_i\} = [M\omega_i^2 + K]^{-1}\{b_i\} = [D_i]^{-1}\{b_i\} \quad (12)$$

The EV approach ensures that the full mode shape produces the estimated dynamic residual when operating on the undamaged model shifted with the experimental modal frequency. However, it should be noted that the active partitions of the mode shapes are not constrained to be the measured values. One contribution of this work is to extend this approach to allow the user to add in these constraints in an incremental process.

The modified approach assumes that the full mode shapes are a linear combination of basis vectors. A modified Ritz vector calculation will be used to generate a set of basis vectors to expand the modes.¹⁶ The Ritz vector calculation procedure provided in Ref. 16 initiates the process by operating on a force vector with the inverse

of the stiffness matrix. The resulting deflection vector is mass normalized and used as the input vector to an inverse iteration with the stiffness matrix inverse and mass matrix as the multipliers. The successive Ritz vectors are orthogonalized with a modified Gram-Schmidt process and mass normalized.

The modified process will generate a set of Ritz-like vectors for each mode of interest using the dynamic residual for each mode as the initial force vector. Two additional modifications to the Ritz vector calculation approach referenced earlier are required. First, to be consistent for perfect data and perfect dynamic residuals, the stiffness matrix is replaced with the dynamic stiffness matrix $[D]$ appropriate for each mode. The next modification is that the mass orthogonalization step includes not just the previously calculated Ritz vectors for that mode but also the previously expanded mode shapes. Hence, a set of basis vectors responding to the forces represented by the dynamic residual and mass orthogonal to themselves and the previously expanded modes of the system will be calculated. Therefore, a linear combination of these basis vectors will be mass orthogonal to the previously expanded modes of the system. These basis vectors will be linearly combined to match the active partition of the mode of interest. Hence for the l th mode of the system the following equations are used:

$$\{\bar{\nu}_1\} = [D_l]^{-1}\{b_l\} \quad (13)$$

$$\{\nu_1\} = \frac{\{\bar{\nu}_1\}}{(\bar{\nu}_1^T M \bar{\nu}_1)^{\frac{1}{2}}} \quad (14)$$

$$\{\bar{\nu}_i\} = [D_l]^{-1}[M]\{\nu_{i-1}\} \quad (15)$$

$$\{\nu_i\} = \{\bar{\nu}_i\} - \sum_{j=1}^{i-1} (\nu_j^T M \bar{\nu}_i) \{\nu_j\} - \sum_{k=1}^{l-1} (\phi_k^T M \bar{\nu}_i) \{\phi_k\} \quad (16)$$

$$(\nu_i^T M \nu_i) = 1 \quad (17)$$

$$\{\phi_i\} = \sum_{i=1}^n \gamma_i \{\nu_i\} \quad (18)$$

This procedure produces a set of expanded mode shapes that are mass orthogonal and consistent with the full dynamic residuals. It is important to note that the first vector is equivalent to the full mode shape calculated with the EV approach given in Eq. (12). The coefficients γ_i in Eq. (18) are determined by partitioning Eq. (18) into the active and omitted sets and then solving a least-squares problem involving only the active set. Hence, by allowing additional terms in the expansion process, the user can trade off confidence in the expanded dynamic residual for confidence in the measured mode shape DOFs.

Damage Location and Extent: MRPT

The estimation of damage location and extent is performed via the MRPT.¹⁷⁻¹⁹ The underlying philosophy of the MRPT is that reduced rank perturbations to the structural matrices are the manifestation of damage. In this approach, damage results in a zero-nonzero pattern in the dynamic residual as given by Eq. (4). Typically, measurement noise and model order reduction or expansion destroy this pattern. Hence, a significant result of this approach is that the zero-nonzero pattern of the dynamic residual will be controlled by the columns of the connectivity matrix that are used to create it.

The estimation of damage extent using a minimum rank formulation utilizes the following calculation (assuming that all damage is manifested only in the stiffness matrix):

$$[\Delta K] = [B][\Phi^T B]^{-1}[B^T] \quad (19)$$

where $[B]$ is the matrix formed of all column vectors of the expanded dynamic residuals for the modes of interest and $[\Phi]$ is the matrix of the expanded modal vectors.

III. Example Application

The example application chosen for this section closely parallels the examples provided in precursory work to provide a context for interpreting the results.^{6,11}

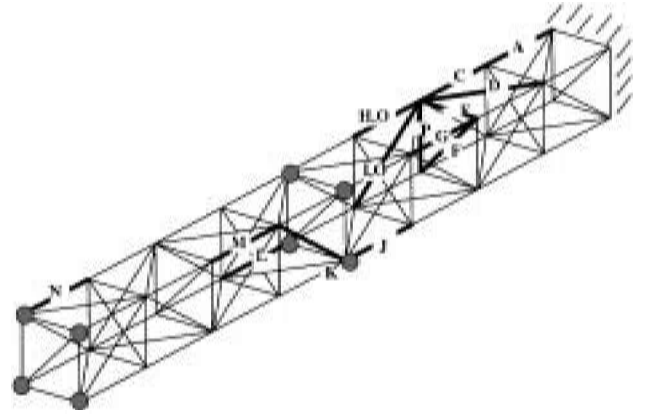


Fig. 1 NASA eight-bay truss structure.

NASA Eight-Bay Truss

For the comparisons in this work the NASA eight-bay truss structure²² is utilized. The eight-bay truss structure is an experimental test article developed to study a variety of damage identification issues. The cantilevered truss includes 32 nodes, each of which was instrumented with triaxial accelerometers. Fifteen unique damage cases were produced by removing individual truss members. The structure and representations of the damage cases (denoted by alphabetic characters) are provided in Fig. 1. This work uses 12 modes from a 96-DOF analytical model of the undamaged structure and the damage cases to explore the developments discussed in Secs. II and III. The DOFs that are assumed to be active or measured for the example provided in this section are denoted by circles in Fig. 1. At these nodes, full triaxial measurements are utilized, resulting in a total measurement set of 24 DOFs. Case D will be used for the examples provided in this section.

Spring Disassembly

The structural matrices for this model include 9216 (96×96) potential entries. However, there are only 320 unique (excluding symmetric entries) nonzero entries in this matrix. After reordering the matrices into analysis and omitted DOFs, the 224 off-diagonal elements are disassembled into 224 springs between DOFs. The 96 diagonal entries are disassembled as springs to ground. Note that this disassembly process does not disallow negative spring stiffness as these springs are usually a simplified representation of a more complicated structural element. It is also important to note that this connectivity pattern becomes an inherent part of subsequent estimations of full residuals, damage location, expanded mode shapes, and damage extent using the procedure outlined in the preceding section.

Selection of Residual Basis Functions

The example presented herein ignored the 96 springs to ground representing the diagonal elements. This enhanced the column linear independence of the down-selected subset. Of the remaining 244 possible basis vectors, fewer than 20 had average MAC values over 0.4. Of these, the six with the highest averaged MAC values were selected. The general rule used to date is to select a number equal to half of the number of modes of interest unless a break in the connectivity of the elements or a rank deficiency (column linear dependence) sets in. Figure 2 provides plots of the six candidate basis functions chosen. These functions do seem to be targeting certain locations on the structure. In fact, the affected DOFs of damage case D (69, 70, 86, and 87) are well represented in these vectors. The first, third, and sixth vectors exclusively relate to these affected DOFs.

Dynamic Residual Reduction/Expansion

The coefficients for the fit of the six basis vectors shown in Fig. 2 to the first dynamic residual using Eq. (11) are $-0.7328, 0, -0.7328, 0, 0$, and 1.4657 . Hence, the first, third, and sixth basis vectors are pulled out as the true components of the full dynamic residual, as indicated in Fig. 3. The true and calculated full residuals for the first

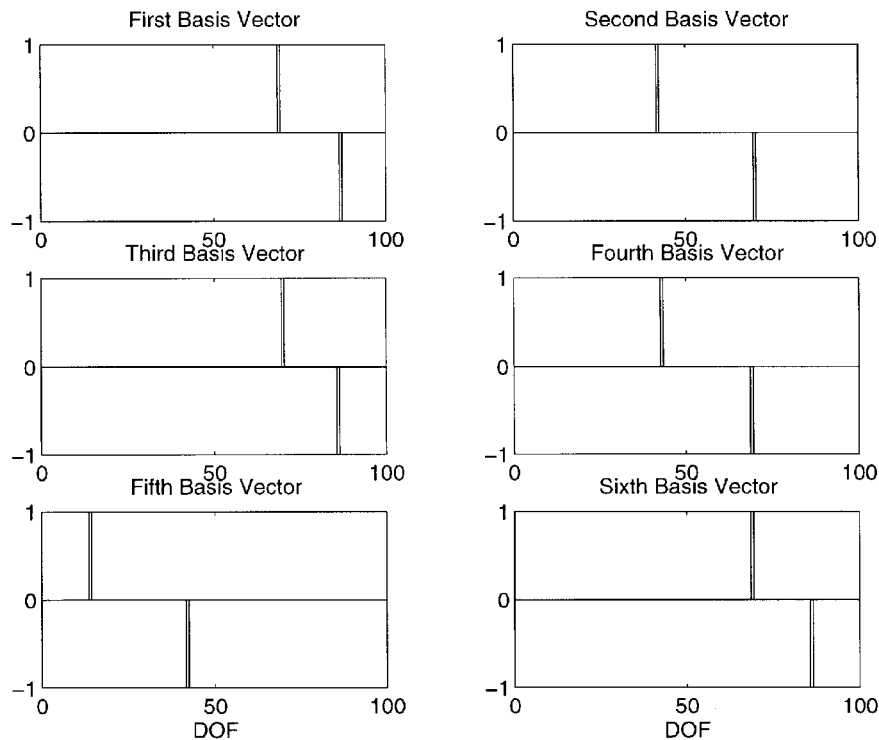


Fig. 2 Six selected basis functions.

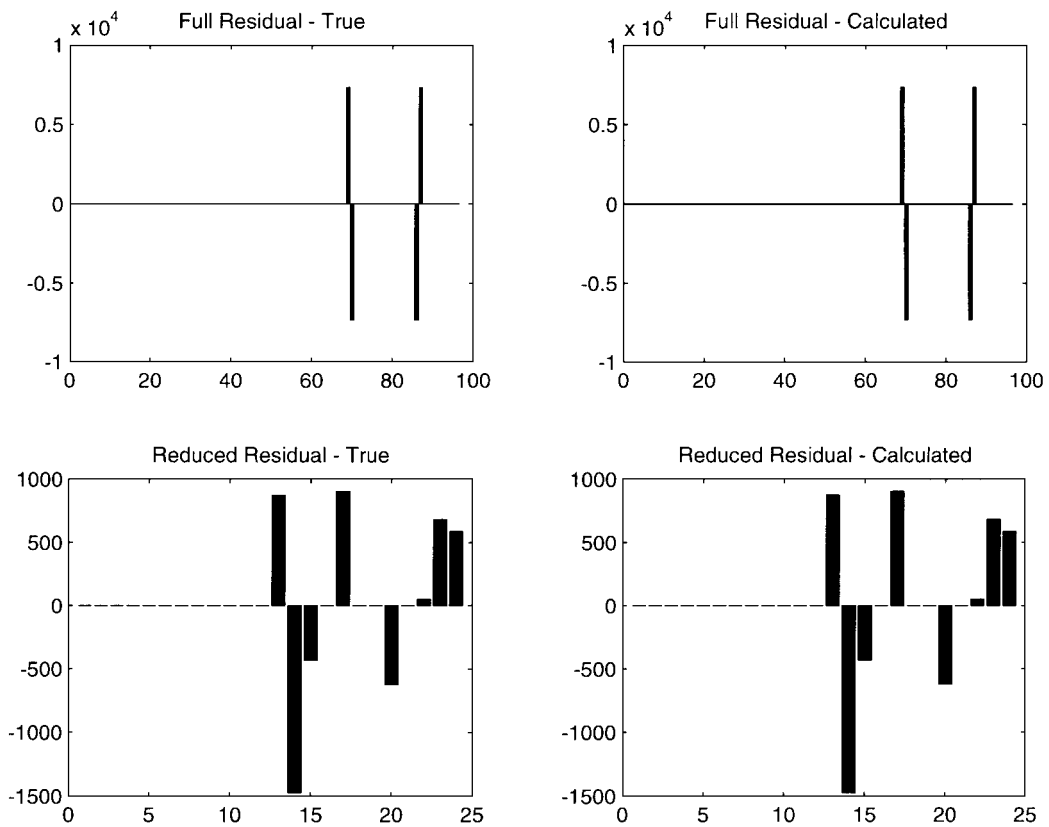


Fig. 3 First mode dynamic residuals.

mode are given in the upper plots of Fig. 3. The true and calculated reduced residuals for the first mode are provided in the lower plots. It is obvious that the correct residuals have been obtained in both cases. However, a more important observation is that the reduced residuals do not maintain the location information in a form that is easily or visually interpreted. Hence, the primary thrust of this work (the estimation of the full residuals) is strongly suggested as important to advancing the field.

Mode Shape Expansion

For the example provided in this section, 12 Ritz-like basis vectors were calculated using Eqs. (13–18). The fact that we are utilizing noise-free measurements produces a situation where the first basis vector exactly matches the expanded mode shape. Figure 4 supports these results for the first mode. The expanded and true mode shapes are provided in the upper plots and are seen to be equivalent. The first and second basis vectors are provided in the lower plots. It is

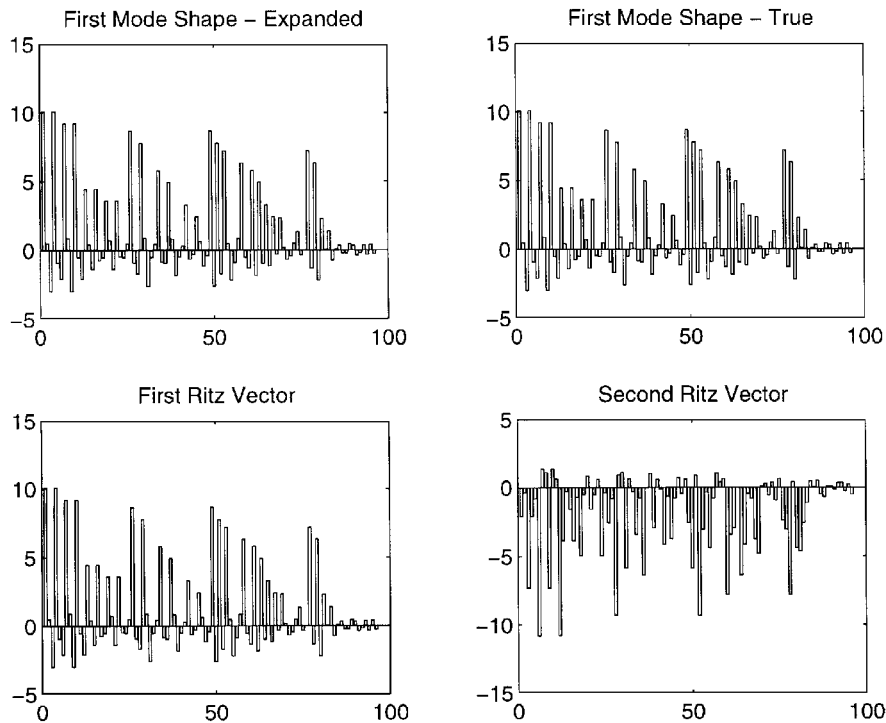


Fig. 4 First expanded mode shape.

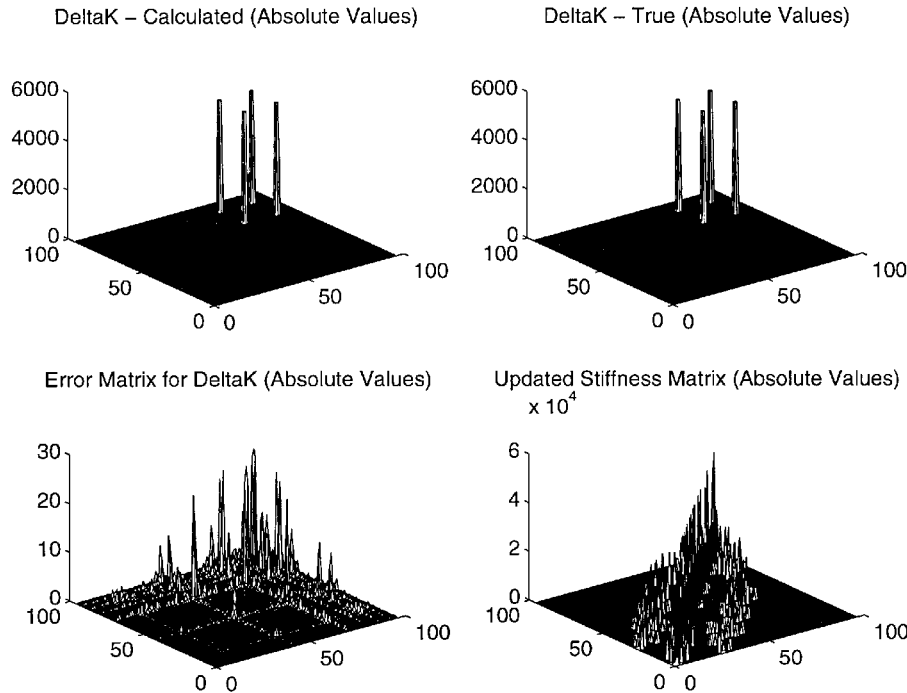


Fig. 5 Damage extent calculation.

obvious that the first basis vector captures the first expanded mode shape. This important point can be used to monitor the consistency of a data set and/or the selection of dynamic residual basis vectors.

Damage Extent

The MRPT update is then calculated using the expanded mode shapes and full dynamic residuals. The upper-left-handplot of Fig. 5 shows the absolute value of this update. The upper-right-handplot shows the true update, which suggests that the update is very close. In fact, the lower-left-hand plot provides the error or difference between the two $[\Delta K]$ matrices. The magnitudes of the differences are at least two orders of magnitude less than the true values. The updated stiffness matrix is provided in the lower-right-handplot as a point of reference. Besides providing an example of the process

developed earlier, this example shows that the process does provide the correct results for perfectly measured data. However, the selection of dynamic residual basis vectors is critical, and the next section will explore the sensitivity of this process.

IV. Sensitivity Issues

This section studies the sensitivity of the preceding procedure to several issues, including the number of residual basis vectors selected, the number of modes used, and the noise in the measurements.

Number of Residual Basis Vectors Used

The most critical part of this coupled analysis is selection of the basis vector set for expansion of the dynamic residuals. The selection

is based on the average MAC value as described in the preceding sections. For the same data case as used earlier, Table 1 provides information on the first 12 potential basis vectors. The average MAC value is provided in column 2. The affected DOFs are in columns 4 and 5. The affected DOFs for this damage case (as well as the correct basis vectors) are denoted by boldface. Based on this information, three observations can be used to decide on the truncation order. First, the MAC values fall off rapidly. After vector 9, the values are less than 0.5. Second, from mode 10 and higher, there is no spatial connection to higher-ranked basis vectors (no common DOFs). This suggests either that multisite damage exists or that these vectors should not belong in the reduced basis set. Finally, the DOFs that appear most often in the first few basis vectors (14, 42, 43, 69, 70, and 86) disappear from the DOF list in lower-order vectors. Other data that can be used to decide on a truncation order include linear independence of the basis vector set, sparsity of the final update matrix, consistency of the dynamic residual from mode to mode, and alignment of the active partitions of the first Ritz vectors with the active partitions of the mode shapes.

Figure 6 summarizes the effects of using different truncation orders on the resulting full dynamic residuals. Orders 3, 6 (as in the preceding section), 9, and 12 are shown. It can be seen that orders 6 and 9 perform very well and 3 and 12 fail. Obviously for the orders that expand the residual vectors appropriately, the correct vectors (196, 197, and 198) dominate the solution. For order 3, the third basis vector needed to complete the fit (no. 196) is not available. This means that the basis vectors do not span the space required to

expand the residual vectors. For the order 12 selection, the least-squares solution produced a higher-rank solution (all basis vectors have nonzero coefficients) that fails to capture the true minimum rank characteristics of the actual damage. This points to the fact that proper selection of the basis vectors will require engineering interaction and judgment similar to that required for experimental modal analysis.

Figure 7 details the effects of basis vector selection on the expanded mode shapes. As with the dynamic residuals, orders 3 and 12 do not perform as well as orders 6 and 9. However, the mode shapes do not have the same sensitivity to the basis vector set. This is due in part to the fact that the mode shapes undergo another fitting process using the Ritz-like vectors. The norm of the error in the update matrix (as provided in the lower-left-hand plot of Fig. 5) provides another indication of the sensitivity to the basis vector selection. The norms of these error vectors are as follows: order 3, 410,000; order 6, 80; order 9, 710; and order 12, 36,000. In summary, the results are extremely sensitive to the truncation order.

Effects of Number of Modes

The effects of the number of modes used in the process are most strongly seen in the residual basis vector selection process. Table 2 details the order of the first six potential basis vectors as a function of the number of modes. Table 1 can be used to interpret most of the DOFs associated with the basis vectors listed in Table 2. The one exception is no. 215, which connects DOF 86 to DOF 93. The important information to be gleaned from this is that the three important vectors for the basis of the dynamic residuals (196, 197, and 198) all appear in the top six positions. In fact, the trend appears to be that more modes tend to move these critical vectors to the top of the list. Also, the process detailed earlier separates out these three

Table 1 MAC ordered residual basis vectors^a

Order	MAC	Vector number	DOF 1	DOF 2
1	0.80	198	69	87
2	0.76	158	42	70
3	0.73	197	70	86
4	0.67	156	43	69
5	0.62	67	14	42
6	0.61	196	69	86
7	0.56	174	44	76
8	0.54	66	13	42
9	0.51	69	14	43
10	0.43	210	74	90
11	0.42	206	75	89
12	0.42	167	46	74

^aThe affected DOFs and the correct basis vectors are denoted by boldface.

Table 2 Effects of number of modes on order of basis vectors^a

Order	6 modes	12 modes	18 modes	24 modes
1	198	198	158	196
2	158	158	197	197
3	67	197	196	198
4	196	156	198	158
5	197	67	67	66
6	69	196	66	215

^aThe correct basis vectors are denoted by boldface.

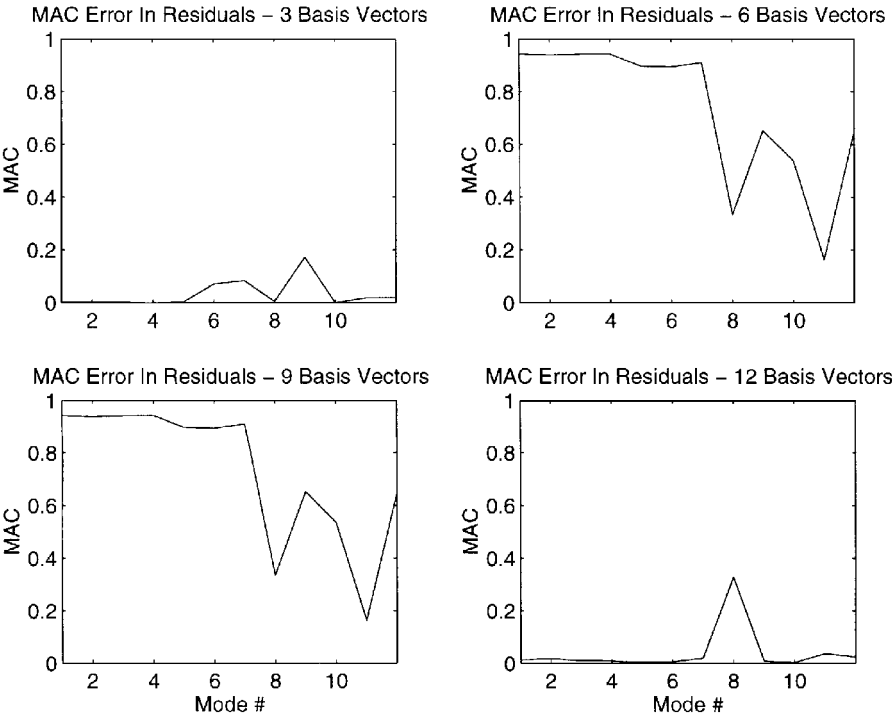


Fig. 6 Effects of basis vector selection dynamic residuals.

vectors if all six of the top vectors are selected with 6, 12, and 18 modes. With 24 modes the numerical process makes incorrect use of basis vector 215. However, the top three basis vectors are all that are needed.

Effects of Noise

To compare the effects of noise, four different sets of random numbers were generated, scaled to 10% of the magnitude of the mode shapes, and added to the mode shapes. Twelve modes were used in the calculations using these noisy modes, and the order of the basis vectors is reported in Table 3. It is important to note that the primary vectors are still typically more highly ranked. Hence damage location is relatively robust with respect to noise. However, damage extent calculations are less robust with respect to noise

unless prefiltering of the shapes or the number of Ritz vectors used in the expansion of the mode shapes is properly controlled.

V. Application to Experimental Data

The coupled damage identification procedure developed earlier was applied to the experimental data for damage case D of the NASA eight-bay truss structure. The data contained five modes with 96 measurement locations as listed earlier. However, only the 24 active DOFs shown in Fig. 1 were used in this example. This represents one of the most difficult active sensor sets due to the lack of even spatial distribution. It was found that not all modes contributed to the selection of the dynamic residual basis vectors. Modes 4 and 5 had almost no ability to discriminate between the vectors. This may be due in part to the analytical model used in the experimental

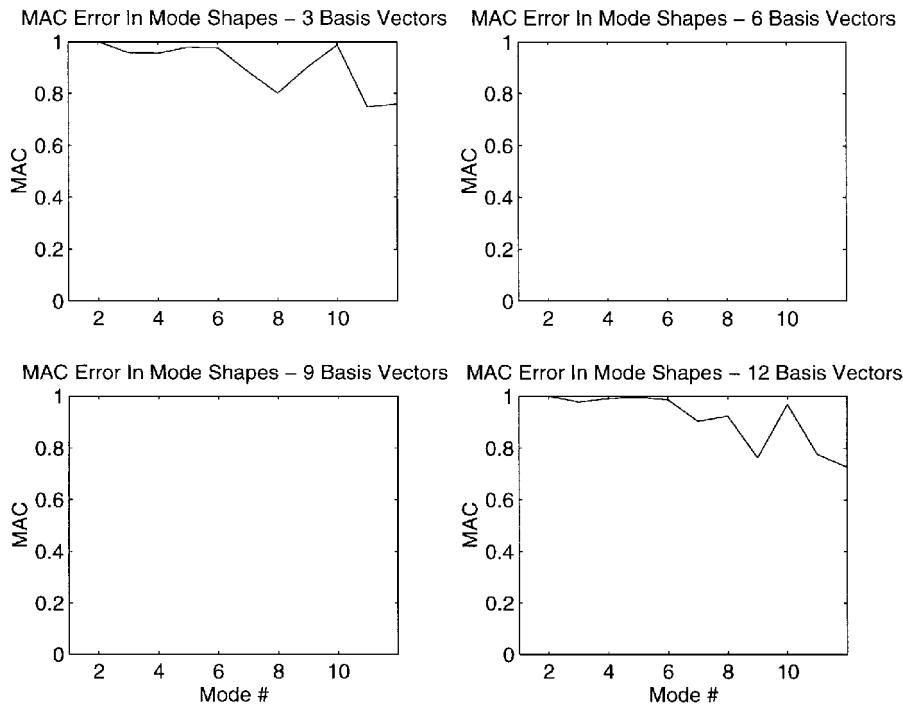


Fig. 7 Effects of basis vector selection on mode shape expansion.

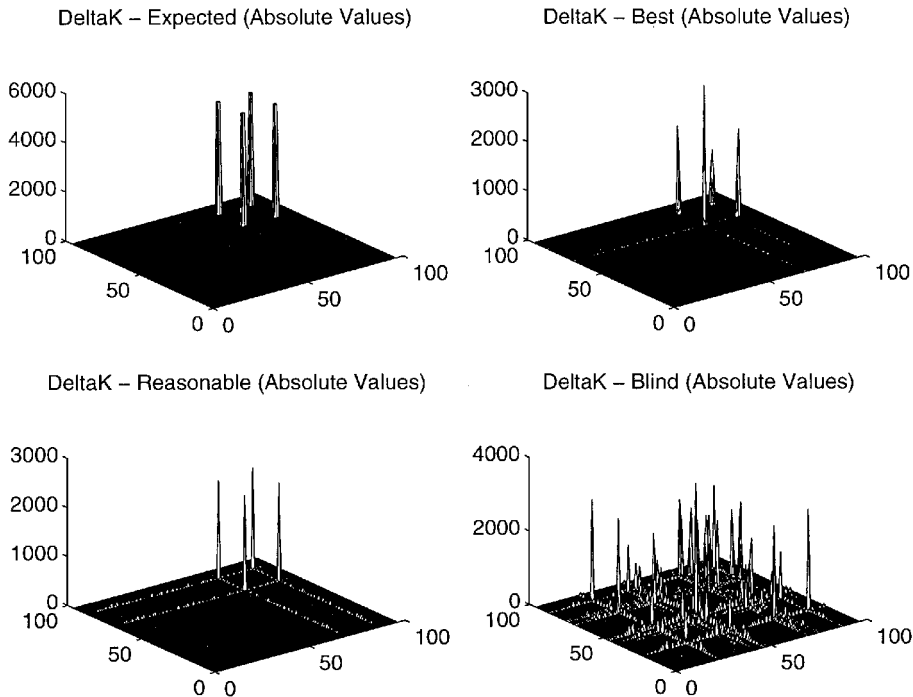


Fig. 8 Experimental update matrices.

Table 3 Effects of noise on order of basis vectors^a

Order	Number 1	Number 2	Number 3	Number 4
1	66	66	198	158
2	196	198	158	198
3	217	156	156	197
4	192	197	197	156
5	198	184	196	174
6	197	158	174	196

^aThe correct basis vectors are denoted by boldface.

Table 4 Order of basis vectors vs number of modes for experimental data

Number of modes	Vector 198	Vector 196	Vector 197
1	1	10	16
2	1	11	16
3	2	18	25
4	2	14	37
5	2	14	38

study. The stiffness matrix was fully populated as a result of the update procedure required to fit the experimental undamaged data. The original sparse stiffness matrix for this system did show these modes as having the ability to discriminate between the vectors. However, the inability of this model to match the undamaged data precluded its use in this study.

Table 4 lists the order of appearance of each of the primary vectors, as determined from the analytical study (196, 197, and 198), as a function of the number of experimental modes used. Vector 198 is consistently high (place 1 or 2). Vector 197 is much lower than in the analytical studies but still reasonably high. Vector 196 was outside the range of reasonable orders to consider.

Figure 8 provides plots of four different manifestations of the model update matrices. The upper-left-hand plot is the analytical update matrix that provides the best estimate of what the experimental results should produce. The upper-right-hand plot provides what could be considered the best estimate of the update matrix if the user knew the results of the previous analytical study and the damage location. For this estimation, the first two modes and basis vectors 198, 197, and 196 were used. The magnitudes are about half of the expected magnitudes, and the sparsity is reasonable. Note that two Ritz-like basis vectors were used to expand the modes. The lower-left-hand plot contains what might be considered a reasonable update for a user with no knowledge of the damage location but plenty of time to work with the data and to develop the proper insight. For this update, the first two modes were used to select the residual basis vectors, and only one basis vector was used—vector 198. The lower-right-hand plot represents a blind application of the process. All five modes were used, and the first six residual basis vectors were selected. Changes were seen at many other locations in the structure. However, the region of the model affected by the damage is dense with large changes. The magnitudes of the changes were roughly equivalent to the other two experimental updates.

VI. Summary and Conclusions

The procedure presented in this work integrates several technologies into an integrated procedure to perform damage location and extent estimation. Of significant importance is the integration of model order reduction and mode shape expansion into the location and extent algorithm. The end result is a hybrid approach that utilizes an analytical model and experimental data to perform rank constrained damage identification. The procedure utilizes minimum rank perturbation theory, spring disassembly, and Ritz vector calculation to couple these technologies. As a result, the issue of determining damage-affected modes is supplanted by determining a truncation order for a set of localized basis vectors. Further work needs to be done to develop experience and techniques for sorting and choosing residual basis vectors.

An application to experimental data proved to be successful in locating the damage. However, this same application pointed out the need to acquire enough modes to discriminate between the residual

basis vectors that drive the rest of the process. The experimental application also suggested that a sparse analytical model could improve the basis vector selection process. Other active sensor sets and damage cases should be exercised to understand the experimental potential of this process more completely.

Acknowledgments

The work of the first author was supported by the State of Texas and the University of Houston through the Institute for Space Systems Operations’s postdoctoral aerospace fellows program. The work of the second author was supported by the Texas Higher Education Coordinating Board Advanced Technology Program under Grant 1-1-19581. The third author was supported by the NASA Johnson Space Center Fellowship Program.

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